

Traffic Engineers and Transport Planners

Our Reference: G19582R-02A

18 January 2019

Greater Shepparton City Council Locked Bag 1000 SHEPPARTON VIC 3632

Attention: Mr Brett Keele

Dear Brett,

Balaclava Road / New Dookie Road / Verney Road / Hawdon Street – Intersection Analysis: Traffic Engineering Assessment

Introduction

Further to your instructions, please find following additional analysis of the Balaclava Road / New Dookie Road / Verney Road / Hawdon Street intersection in Shepparton.

Traffix Group was engaged in 2015 by Greater Shepparton City Council to undertake a traffic assessment on a number of intersection upgrade options at Balaclava Road / New Dookie Road / Verney Road / Hawdon Street in Shepparton including traffic signals and an upgraded roundabout (Our Ref: G19582R-01C dated 26/4/2016).

Each of the intersection options was assessed in a present scenario (2015) and a future scenario (2026 design year) and a functional layout plan was developed for the recommended traffic signal option.

We understand that Council is seeking additional SIDRA analysis to compare the performance of the recommended traffic signals with the existing roundabout. The following letter provides a comparison of existing roundabout layout and the proposed traffic signals to inform stakeholder consultation.

Comparison Scenarios

The following scenarios to compare the proposed traffic signals and the existing roundabout have been assessed:

- Scenario 1: existing roundabout ('0 years') compared to the existing roundabout ('10 years'), and
- Scenario 2: existing roundabout ('10 years') compared to the proposed traffic signals ('10 years').

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Traffic Volume Data Sets

The traffic volumes utilised for this assessment were sourced from our previous report (Our Ref: G19582R-01C dated 26/4/2016). The key traffic volume scenarios are as follows:

- '0 years' scenarios: Turning movement count data collected at the intersection in 2015.
- **'10 years' scenarios:** Data set developed from the 2015 turning movement count data with additional volumes associated with likely developments in the area and a 2% growth factor for general growth on the road network. This data set represents a horizon year of 2026 (i.e. 10 years beyond 2015).

A full summary of the development of the future traffic volume data set can be found in our previous report.

Intersection Layouts

The SIDRA layouts adopted for this analysis are shown in Figure 1 and Figure 2.

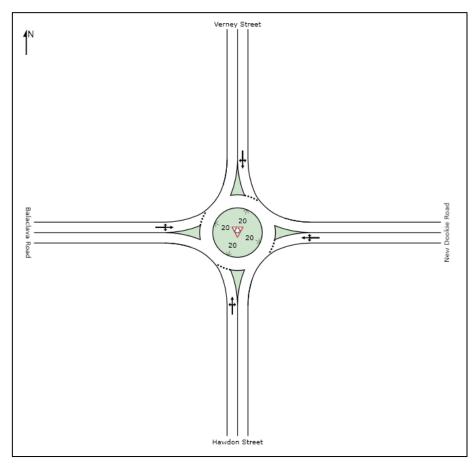


Figure 1: Existing Roundabout Layout



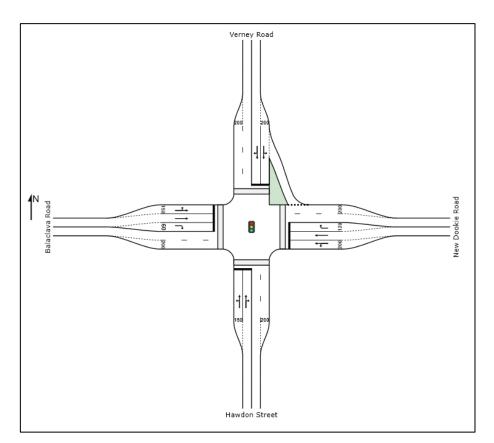


Figure 2: Proposed Traffic Signals

SIDRA Results - Scenario 1

Scenario 1 generally represents a 'Do Nothing' case where the existing roundabout layout is retained and the intersection is modelled with estimated traffic volumes ten (10) years into the future (i.e. 2026)

Table 1 below compares the existing roundabout at Balaclava Road / New Dookie Road / Verney Road under a '0 Year' scenario and a '10 Year' scenario.

		AM Peak			PM Peak	
	Degree of Saturation	Average Delay (s)	Queue Length (m)	Degree of Saturation	Average Delay (s)	Queue Length (m)
South Approa	ch: Hawdon Stre	eet				
0 Year	0.53	10	32	0.88	30	114
10 Year	1.41	394	1,206	1.30	291	1,086
Difference	+0.88	+384	+1,174	+0.42	+261	+972

Table 1: Scenario 1 Comparison



Balaclava Road / New Dookie Road / Verney Road / Hawdon Street

		AM Peak			PM Peak	
	Degree of Saturation	Average Delay (s)	Queue Length (m)	Degree of Saturation	Average Delay (s)	Queue Length (m)
East Approach	: New Dookie R	oad				
0 Year	0.58	12	40	0.94	37	167
10 Year	0.94	34	185	2.03	947	2,829
Difference	+0.36	+22	+145	+1.09	+910	+2,662
North Approa	ch: Verney Stree	et				
0 Year	0.71	13	60	0.71	12	58
10 Year	1.65	611	1,738	0.99	46	248
Difference	+0.94	+598	+1,678	+0.28	+34	+190
West Approac	h: Balaclava Roa	ad				
0 Year	0.57	11	42	0.97	64	158
10 Year	1.24	244	858	1.84	786	1,463
Difference	+0.67	+233	+816	+0.87	+722	+1,035

Table 1 shows that the performance of the existing roundabout severely deteriorates in the ten (10) year period following the existing conditions assessment with all of the approaches experiencing a degree of saturation above 1.0 in at least one peak period. This leads to SIDRA to estimate long delays and long queue lengths.

We note that the SIDRA estimates of delay and queue length can become less precise as the degree of saturation exceeds 1.0. On this basis, the specific delays and queue lengths are may not be representative of the actual intersection performance and should only be used as an indication that the intersection is operating well above capacity. Nevertheless, the SIDRA model clearly indicates that the existing roundabout layout will experience major traffic congestion prior to the 2026 design year with three of the four approaches experiencing average delays in excess of five minutes and maximum queues exceeding 500m.

A copy of the SIDRA results for Scenario 1 are provided in Appendix A and B.



SIDRA Results - Scenario 2

Scenario 2 compares the existing roundabout layout and the proposed traffic signal layout with the estimated traffic volumes ten (10) years into the future. Table 2 below summarises the SIDRA results for Scenario 2.

Table 2: Scenario 2 Comparison

		AM Peak			PM Peak	
	Degree of Saturation	Average Delay (s)	Queue Length (m)	Degree of Saturation	Average Delay (s)	Queue Length (m)
South Approa	ch: Hawdon Stre	eet				
Roundabout	1.41	394	1,206	1.30	291	1,086
Signals	0.84	60	130	0.89	61	185
Difference	-0.57	-334	-1,076	-0.41	-230	-901
East Approach	: New Dookie R	oad				
Roundabout	0.94	34	185	2.03	947	2,829
Signals	0.86	53	92	0.91	56	167
Difference	-0.08	-19	-93	-1.12	-891	-2,662
North Approa	ch: Verney Stree	et				
Roundabout	1.65	611	1,738	0.99	46	248
Signals	0.85	53	195	0.88	61	175
Difference	-0.80	-558	-1,543	-0.11	+15	-73
West Approac	h: Balaclava Roa	ad				
Roundabout	1.24	244	858	1.84	786	1,463
Signals	0.86	59	170	0.91	71	124
Difference	-0.38	-185	-688	-0.93	-715	-1,339

Table 2 above shows the in the '10 Year' scenario, the construction of the traffic signals results in significant improvements along all approaches from the existing roundabout layout.

As mentioned previously, SIDRA is less accurate as the degree of saturation exceeds 1.0. On this basis, it is difficult to quantify the exact improvements in maximum queue length and average delay, however, the 'Do Nothing' scenario will result in at least three of the four approaches experiencing major delays greater than five minutes and queue lengths that exceed 500m.

Under a '10 Year' scenario the traffic signals operate satisfactorily with a maximum queue length of 195m along the north approach in the AM peak and a maximum average delay of 71 seconds along the west approach in the PM peak.

A copy of the SIDRA results for Scenario 2 are provided in Appendix B and C.



SIDRA Detailed Outputs

The detailed SIDRA outputs for each option are provided in Appendix A to Appendix C. For each option the following summaries are provided:

- Intersection layout
- Movement summary
- Lane summary
- Phasing summary (where applicable for traffic signal options only).

Conclusions

Based on the above assessments, the existing roundabout at Balaclava Road / New Dookie Road / Verney Road / Hawdon Street will continue to deteriorate and operate significantly above capacity in the future '10 Year' scenario. This will result in three of the four approaches experiencing major delays in excess of five minutes and queue lengths greater than 500m.

The construction of traffic signals will result in significantly improved performance of the intersection in the future scenario with a maximum average delays of 71 seconds and maximum queue lengths less than 200m.

Please contact myself at Traffix Group if you require any further information.

Yours faithfully, TRAFFIX GROUP PTY LTD

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Appendix A – Roundabout '0 Years'

Appendix B – Roundabout '10 Years'

Appendix C – Traffic Signals '10 Years'



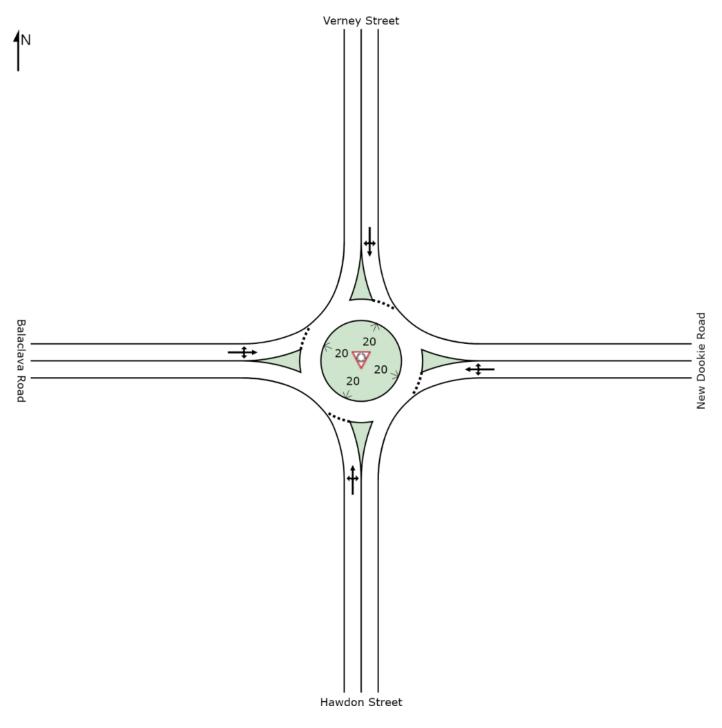
Appendix A: Roundabout '0 Years'

SITE LAYOUT

Site: Balaclava Road / New Dookie Road - AM Existing

Shepparton

Roundabout



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₩ Site: Balaclava Road / New Dookie Road - AM Existing

Shepparton

Roundabout

Move	ment Per	formance - V	/ehicles								
Mov	OD	Demand		Deg.	Average	Level of	95% Back		Prop.	Effective	Average
ID	Mov	Total veh/h	HV %	Satn v/c	Delay	Service	Vehicles veh	Distance	Queued	Stop Rate	Speed
South	: Hawdon S		70	V/C	sec	_	ven	m	_	per veh	km/h
1	L2	66	1.6	0.530	9.0	LOS A	4.4	31.9	0.81	0.87	51.0
2	T1	266	5.5	0.530	9.4	LOS A	4.4	31.9	0.81	0.87	52.1
3	R2	66	3.2	0.530	13.9	LOS B	4.4	31.9	0.81	0.87	52.0
Appro	ach	399	4.5	0.530	10.1	LOS B	4.4	31.9	0.81	0.87	51.9
East: I	New Dookie	e Road									
4	L2	67	6.3	0.582	9.8	LOS A	5.4	40.3	0.85	0.93	49.8
5	T1	236	8.9	0.582	10.2	LOS B	5.4	40.3	0.85	0.93	50.9
6	R2	148	8.5	0.582	14.8	LOS B	5.4	40.3	0.85	0.93	50.8
Appro	ach	452	8.4	0.582	11.6	LOS B	5.4	40.3	0.85	0.93	50.7
North:	Verney Str	reet									
7	L2	168	5.6	0.713	12.2	LOS B	8.4	60.3	0.91	1.00	49.0
8	T1	308	2.7	0.713	12.2	LOS B	8.4	60.3	0.91	1.00	50.1
9	R2	78	2.7	0.713	16.9	LOS B	8.4	60.3	0.91	1.00	50.1
Appro	ach	555	3.6	0.713	12.9	LOS B	8.4	60.3	0.91	1.00	49.8
West:	Balaclava	Road									
10	L2	111	27.6	0.571	10.4	LOS B	5.0	42.1	0.83	0.92	49.7
11	T1	231	14.6	0.571	10.0	LOS B	5.0	42.1	0.83	0.92	51.2
12	R2	94	37.1	0.571	15.7	LOS B	5.0	42.1	0.83	0.92	50.2
Appro	ach	435	22.8	0.571	11.3	LOS B	5.0	42.1	0.83	0.92	50.6
All Vel	nicles	1840	9.5	0.713	11.6	LOS B	8.4	60.3	0.86	0.94	50.6

Level of Service (LOS) Method: Delay (HCM 2000).

Roundabout LOS Method: Same as Signalised Intersections.

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Shepparton

Roundabout

Lane Use a	nd Perfo	rmanc	e:										
	Demand		Con	Deg.	Lane	Average	Level of	95% Back of		Lane	Lane	Cap.	Prob.
	Total veh/h	HV %	Cap. veh/h	Satn v/c	Util. %	Delay sec	Service	Veh	Dist m	Config	Length m	Adj. %	Block. %
South: Hawd		70	Veniim		/0	000						/0	/0
Lane 1 ^d	399	4.5	753	0.530	100	10.1	LOS B	4.4	31.9	Full	500	0.0	0.0
Approach	399	4.5		0.530		10.1	LOS B	4.4	31.9				
East: New Do	ookie Road	b											
Lane 1 ^d	452	8.4	776	0.582	100	11.6	LOS B	5.4	40.3	Full	500	0.0	0.0
Approach	452	8.4		0.582		11.6	LOS B	5.4	40.3				
North: Verne	y Street												
Lane 1 ^d	555	3.6	778	0.713	100	12.9	LOS B	8.4	60.3	Full	500	0.0	0.0
Approach	555	3.6		0.713		12.9	LOS B	8.4	60.3				
West: Balacla	ava Road												
Lane 1 ^d	435	22.8	762	0.571	100	11.3	LOS B	5.0	42.1	Full	500	0.0	0.0
Approach	435	22.8		0.571		11.3	LOS B	5.0	42.1				
Intersection	1840	9.5		0.713		11.6	LOS B	8.4	60.3				

Level of Service (LOS) Method: Delay (HCM 2000).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay per lane.

Intersection and Approach LOS values are based on average delay for all lanes.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

d Dominant lane on roundabout approach

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₩ Site: Balaclava Road / New Dookie Road - PM Existing

Shepparton

Roundabout

Move	ment Per	formance - V	/ehicles								
Mov ID	OD Mov	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back (Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South	Hawdon S	Street									
1	L2	84	2.5	0.883	28.9	LOS C	15.9	113.8	1.00	1.43	40.2
2	T1	396	1.6	0.883	29.0	LOS C	15.9	113.8	1.00	1.43	41.0
3	R2	52	14.3	0.883	34.5	LOS C	15.9	113.8	1.00	1.43	40.6
Approa	ach	532	3.0	0.883	29.6	LOS C	15.9	113.8	1.00	1.43	40.8
East: I	New Dookie	e Road									
4	L2	75	1.4	0.940	34.8	LOS C	23.3	166.9	1.00	1.61	37.3
5	T1	315	4.0	0.940	35.2	LOS D	23.3	166.9	1.00	1.61	37.9
6	R2	258	1.2	0.940	39.6	LOS D	23.3	166.9	1.00	1.61	37.9
Appro	ach	647	2.6	0.940	36.9	LOS D	23.3	166.9	1.00	1.61	37.9
North:	Verney Str	eet									
7	L2	155	0.0	0.705	11.3	LOS B	8.2	58.4	0.91	0.98	49.6
8	T1	328	1.3	0.705	11.6	LOS B	8.2	58.4	0.91	0.98	50.7
9	R2	69	9.1	0.705	16.6	LOS B	8.2	58.4	0.91	0.98	50.4
Approa	ach	553	1.9	0.705	12.1	LOS B	8.2	58.4	0.91	0.98	50.3
West:	Balaclava	Road									
10	L2	67	3.1	0.971	62.3	LOS E	21.6	157.5	1.00	1.81	29.3
11	T1	226	7.4	0.971	63.0	LOS E	21.6	157.5	1.00	1.81	29.7
12	R2	121	0.0	0.971	66.9	LOS E	21.6	157.5	1.00	1.81	29.7
Appro	ach	415	4.6	0.971	64.0	LOS E	21.6	157.5	1.00	1.81	29.6
All Vel	nicles	2146	2.9	0.971	33.9	LOS C	23.3	166.9	0.98	1.44	38.9

Level of Service (LOS) Method: Delay (HCM 2000).

Roundabout LOS Method: Same as Signalised Intersections.

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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𝒞 Site: Balaclava Road / New Dookie Road - PM Existing

Shepparton

Roundabout

Lane Use a	nd Perfor	manc	e:										
	Demand F		Con	Deg.	Lane	Average	Level of	95% Back o		Lane	Lane	Cap.	Prob.
	Total veh/h	HV %	Cap. veh/h	Satn v/c	Util. %	Delay sec	Service	Veh	Dist m	Config	Length m	Adj. %	Block. %
South: Hawde	-	70	Veniim		/0	000						70	/0
Lane 1 ^d	532	3.0	602	0.883	100	29.6	LOS C	15.9	113.8	Full	500	0.0	0.0
Approach	532	3.0		0.883		29.6	LOS C	15.9	113.8				
East: New Do	okie Road												
Lane 1 ^d	647	2.6	689	0.940	100	36.9	LOS D	23.3	166.9	Full	500	0.0	0.0
Approach	647	2.6		0.940		36.9	LOS D	23.3	166.9				
North: Verney	/ Street												
Lane 1 ^d	553	1.9	784	0.705	100	12.1	LOS B	8.2	58.4	Full	500	0.0	0.0
Approach	553	1.9		0.705		12.1	LOS B	8.2	58.4				
West: Balacla	ava Road												
Lane 1 ^d	415	4.6	427	0.971	100	64.0	LOS E	21.6	157.5	Full	500	0.0	0.0
Approach	415	4.6		0.971		64.0	LOS E	21.6	157.5				
Intersection	2146	2.9		0.971		33.9	LOS C	23.3	166.9				

Level of Service (LOS) Method: Delay (HCM 2000).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay per lane.

Intersection and Approach LOS values are based on average delay for all lanes.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

d Dominant lane on roundabout approach

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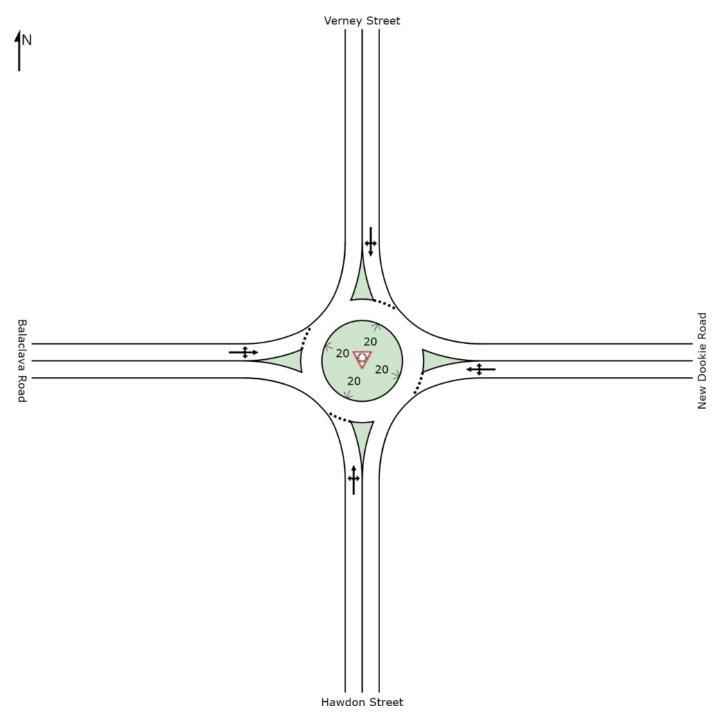
Appendix B: Roundabout '10 Years'

SITE LAYOUT

Site: Balaclava Road / New Dookie Road - AM Future

Shepparton

Roundabout



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😵 Site: Balaclava Road / New Dookie Road - AM Future

Shepparton

Roundabout

Move	ment Per	formance - V	/ehicles								
Mov ID	OD Mov	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South:	Hawdon S	Street									
1	L2	80	3.9	1.411	391.7	LOS F	166.0	1205.7	1.00	5.99	8.2
2	T1	369	4.6	1.411	392.0	LOS F	166.0	1205.7	1.00	5.99	8.2
3	R2	361	4.4	1.411	396.6	LOS F	166.0	1205.7	1.00	5.99	8.2
Approa	ach	811	4.4	1.411	394.0	LOS F	166.0	1205.7	1.00	5.99	8.2
East: N	New Dookie	e Road									
4	L2	142	8.1	0.942	32.3	LOS C	24.7	185.3	1.00	1.58	38.4
5	T1	369	8.3	0.942	32.6	LOS C	24.7	185.3	1.00	1.58	39.1
6	R2	208	8.6	0.942	37.2	LOS D	24.7	185.3	1.00	1.58	39.0
Approa	ach	720	8.3	0.942	33.9	LOS C	24.7	185.3	1.00	1.58	38.9
North:	Verney Str	reet									
7	L2	245	3.4	1.654	610.1	LOS F	240.8	1738.3	1.00	7.57	5.5
8	T1	523	3.6	1.654	610.3	LOS F	240.8	1738.3	1.00	7.57	5.5
9	R2	132	4.0	1.654	615.0	LOS F	240.8	1738.3	1.00	7.57	5.5
Approa	ach	900	3.6	1.654	610.9	LOS F	240.8	1738.3	1.00	7.57	5.5
West:	Balaclava	Road									
10	L2	157	22.8	1.238	243.4	LOS F	102.6	857.7	1.00	4.68	12.1
11	T1	417	22.7	1.238	243.6	LOS F	102.6	857.7	1.00	4.68	12.1
12	R2	122	22.4	1.238	248.3	LOS F	102.6	857.7	1.00	4.68	12.1
Approa	ach	696	22.7	1.238	244.4	LOS F	102.6	857.7	1.00	4.68	12.1
All Veh	nicles	3126	9.2	1.654	340.2	LOS F	240.8	1738.3	1.00	5.14	9.3

Level of Service (LOS) Method: Delay (HCM 2000).

Roundabout LOS Method: Same as Signalised Intersections.

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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𝒞 Site: Balaclava Road / New Dookie Road - AM Future

Shepparton

Roundabout

Lane Use a	nd Perfo	rmanc	e:										
	Demand Total veh/h	Flows HV %	Cap. veh/h	Deg. Satn v/c	Lane Util. %	Average Delay sec	Level of Service	95% Back Veh	of Queue Dist m	Lane Config	Lane Length m	Cap. Adj. %	Prob. Block. %
South: Hawdo	on Street												
Lane 1 ^d	811	4.4	574	1.411	100	394.0	LOS F	166.0	1205.7	Full	500	0.0	<mark>46.6</mark>
Approach	811	4.4		1.411		394.0	LOS F	166.0	1205.7				
East: New Do	ookie Road	d											
Lane 1 ^d	720	8.3	765	0.942	100	33.9	LOS C	24.7	185.3	Full	500	0.0	0.0
Approach	720	8.3		0.942		33.9	LOS C	24.7	185.3				
North: Verney	/ Street												
Lane 1 ^d	900	3.6	544	1.654	100	610.9	LOS F	240.8	1738.3	Full	500	0.0	<mark>100.0</mark>
Approach	900	3.6		1.654		610.9	LOS F	240.8	1738.3				
West: Balacla	ava Road												
Lane 1 ^d	696	22.7	562	1.238	100	244.4	LOS F	102.6	857.7	Full	500	0.0	<mark>24.4</mark>
Approach	696	22.7		1.238		244.4	LOS F	102.6	857.7				
Intersection	3126	9.2		1.654		340.2	LOS F	240.8	1738.3				

Level of Service (LOS) Method: Delay (HCM 2000).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay per lane.

Intersection and Approach LOS values are based on average delay for all lanes.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

d Dominant lane on roundabout approach

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𝒞 Site: Balaclava Road / New Dookie Road - PM Future

Shepparton

Roundabout

Move	ment Per	formance - V	ehicles								
Mov ID	OD Mov	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South:	Hawdon S	Street								· ·	
1	L2	101	3.1	1.299	289.8	LOS F	151.2	1085.5	1.00	5.50	10.5
2	T1	604	3.0	1.299	290.1	LOS F	151.2	1085.5	1.00	5.50	10.6
3	R2	203	3.1	1.299	294.7	LOS F	151.2	1085.5	1.00	5.50	10.6
Approa	ach	908	3.0	1.299	291.1	LOS F	151.2	1085.5	1.00	5.50	10.6
East: N	New Dookie	e Road									
4	L2	260	2.4	2.031	945.1	LOS F	395.7	2829.0	1.00	9.47	3.7
5	T1	536	2.6	2.031	945.3	LOS F	395.7	2829.0	1.00	9.47	3.7
6	R2	384	2.5	2.031	949.9	LOS F	395.7	2829.0	1.00	9.47	3.7
Approa	ach	1180	2.5	2.031	946.8	LOS F	395.7	2829.0	1.00	9.47	3.7
North:	Verney Str	reet									
7	L2	207	2.0	0.987	45.2	LOS D	34.8	247.9	1.00	1.84	34.1
8	T1	466	1.8	0.987	45.4	LOS D	34.8	247.9	1.00	1.84	34.7
9	R2	101	2.1	0.987	50.0	LOS D	34.8	247.9	1.00	1.84	34.6
Approa	ach	775	1.9	0.987	45.9	LOS D	34.8	247.9	1.00	1.84	34.5
West:	Balaclava	Road									
10	L2	120	4.4	1.842	784.2	LOS F	201.2	1463.3	1.00	6.48	4.4
11	T1	369	4.6	1.842	784.5	LOS F	201.2	1463.3	1.00	6.48	4.4
12	R2	158	4.7	1.842	789.1	LOS F	201.2	1463.3	1.00	6.48	4.4
Approa	ach	647	4.6	1.842	785.6	LOS F	201.2	1463.3	1.00	6.48	4.4
All Veh	nicles	3511	2.9	2.031	548.6	LOS F	395.7	2829.0	1.00	6.21	6.1

Level of Service (LOS) Method: Delay (HCM 2000).

Roundabout LOS Method: Same as Signalised Intersections.

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

The results of iterative calculations indicate a somewhat unstable solution. See the Diagnostics section in the Detailed Output report.

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😵 Site: Balaclava Road / New Dookie Road - PM Future

Shepparton

Roundabout

Lane Use a	nd Perfor	manc	e										
	Demand F Total veh/h	lows HV %	Cap. veh/h	Deg. Satn v/c	Lane Util. %	Average Delay sec	Level of Service	95% Back Veh	of Queue Dist m	Lane Config	Lane Length m	Cap. Adj. %	Prob. Block. %
South: Hawde	on Street												
Lane 1 ^d	908	3.0	699	1.299	100	291.1	LOS F	151.2	1085.5	Full	500	0.0	<mark>37.7</mark>
Approach	908	3.0		1.299		291.1	LOS F	151.2	1085.5				
East: New Do	ookie Road												
Lane 1 ^d	1180	2.5	581	2.031	100	946.8	LOS F	395.7	2829.0	Full	500	0.0	<mark>100.0</mark>
Approach	1180	2.5		2.031		946.8	LOS F	395.7	2829.0				
North: Verney	/ Street												
Lane 1 ^d	775	1.9	785	0.987	100	45.9	LOS D	34.8	247.9	Full	500	0.0	0.0
Approach	775	1.9		0.987		45.9	LOS D	34.8	247.9				
West: Balacla	ava Road												
Lane 1 ^d	647	4.6	351	1.842	100	785.6	LOS F	201.2	1463.3	Full	500	0.0	<mark>95.6</mark>
Approach	647	4.6		1.842		785.6	LOS F	201.2	1463.3				
Intersection	3511	2.9		2.031		548.6	LOS F	395.7	2829.0				

Level of Service (LOS) Method: Delay (HCM 2000).

Roundabout LOS Method: Same as Signalised Intersections.

Lane LOS values are based on average delay per lane.

Intersection and Approach LOS values are based on average delay for all lanes.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

The results of iterative calculations indicate a somewhat unstable solution. See the Diagnostics section in the Detailed Output report.

d Dominant lane on roundabout approach

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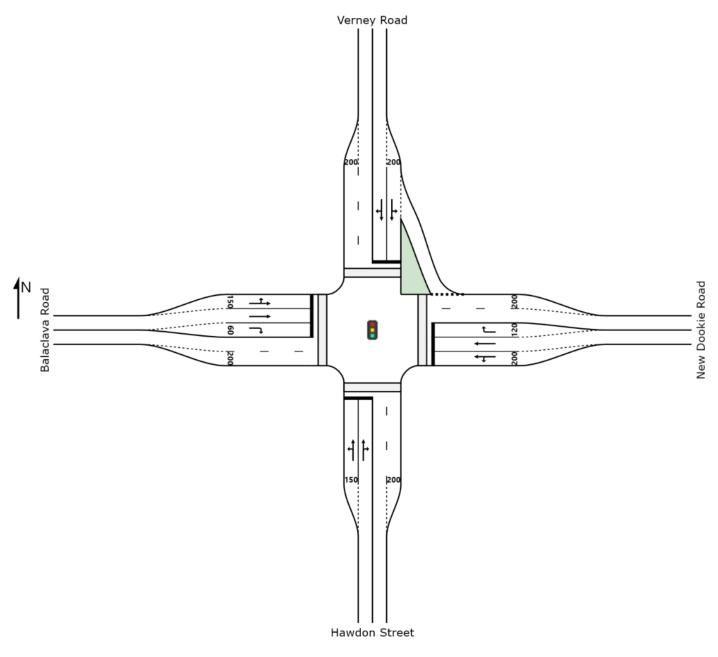


Appendix C: Traffic Signals '10 Years'

SITE LAYOUT

Site: Traffic Signals - AM

New Site Signals - Fixed Time Isolated



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Site: Traffic Signals - AM

New Site

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time)

	ment Perfo										
Mov ID	OD Mov	Demano Total	d Flows HV	Deg. Satn	Average	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective	Average
טו	IVIOV	veh/h	пv %	v/c	Delay sec	Service	venicies veh	Distance	Queueu	Stop Rate per veh	Speed km/t
South	: Hawdon St		,,,				Von				
1	L2	80	1.6	0.839	63.7	LOS E	17.9	130.3	1.00	0.97	29.9
2	T1	369	5.5	0.839	58.1	LOS E	17.9	130.3	1.00	0.97	30.3
3	R2	123	3.2	0.839	63.8	LOS E	17.8	129.4	1.00	0.97	29.7
Appro	ach	573	4.5	0.839	60.1	LOS E	17.9	130.3	1.00	0.97	30.1
East: I	New Dookie	Road									
4	L2	92	6.3	0.545	50.3	LOS D	11.6	86.9	0.94	0.80	33.3
5	T1	357	8.9	0.545	44.7	LOS D	11.8	88.9	0.94	0.79	34.4
6	R2	188	8.5	0.861	71.0	LOS E	12.3	92.1	1.00	0.94	27.3
Appro	ach	637	8.4	0.861	53.3	LOS D	12.3	92.1	0.95	0.84	31.8
North:	Verney Roa	d									
7	L2	245	5.6	0.849	55.5	LOS E	26.9	195.2	1.00	1.05	32.0
8	T1	523	2.7	0.849	51.1	LOS D	26.9	195.2	1.00	1.01	32.1
9	R2	132	2.7	0.849	57.6	LOS E	26.2	187.6	1.00	0.97	31.5
Appro	ach	900	3.5	0.849	53.2	LOS D	26.9	195.2	1.00	1.01	32.0
West:	Balaclava R	oad									
10	L2	145	27.6	0.860	63.7	LOS E	20.7	170.0	1.00	0.99	29.4
11	T1	479	14.6	0.860	57.2	LOS E	20.7	170.0	0.99	1.00	30.7
12	R2	113	37.1	0.613	62.3	LOS E	6.6	60.6	1.00	0.81	28.9
Appro	ach	737	20.6	0.860	59.2	LOS E	20.7	170.0	1.00	0.97	30.2
All Vel	hicles	2846	9.2	0.861	56.2	LOS E	26.9	195.2	0.99	0.95	31.

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Mov		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P1	South Full Crossing	53	44.3	LOS E	0.2	0.2	0.86	0.86
P2	East Full Crossing	53	42.6	LOS E	0.2	0.2	0.84	0.84
P3	North Full Crossing	53	44.3	LOS E	0.2	0.2	0.86	0.86
P4	West Full Crossing	53	51.5	LOS E	0.2	0.2	0.93	0.93
All Pe	destrians	211	45.7	LOS E			0.87	0.87

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: Traffic Signals - AM

New Site

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time)

Lano Lleo d	and Porfo	rmana	•										
Lane Use and Performance Demand Flows Deg. Lane Average Level of 95% Back of Que											Lane	Cap.	Prob.
	Total	HV	Cap.	Satn	Util.	Delay	Service	Veh	Dist	Lane Config	Length	Adj.	Block.
	veh/h	%	veh/h	v/c	%	sec			m		m	%	%
South: Hawo	on Street												
Lane 1	288	4.4	343	0.839	100	59.7	LOS E	17.9	130.3	Short	150	0.0	NA
Lane 2	285	4.5	340	0.839	100	60.6	LOS E	17.8	129.4	Full	500	0.0	0.0
Approach	573	4.5		0.839		60.1	LOS E	17.9	130.3				
East: New Dookie Road													
Lane 1	223	7.8	409	0.545	100	47.0	LOS D	11.6	86.9	Short	200	0.0	NA
Lane 2	226	8.9	415	0.545	100	44.7	LOS D	11.8	88.9	Full	500	0.0	0.0
Lane 3	188	8.5	219	0.861	100	71.0	LOS E	12.3	92.1	Short	120	0.0	NA
Approach	637	8.4		0.861		53.3	LOS D	12.3	92.1				
North: Verne	ey Road												
Lane 1	473	4.2	557	0.849	100	52.7	LOS D	26.9	195.2	Short	200	0.0	NA
Lane 2	427	2.7	503	0.849	100	53.8	LOS D	26.2	187.6	Full	500	0.0	0.0
Approach	900	3.5		0.849		53.2	LOS D	26.9	195.2				
West: Balacl	ava Road												
Lane 1	323	20.4	376	0.860	100	60.5	LOS E	20.7	170.0	Short	150	0.0	NA
Lane 2	301	14.6	350 ¹	0.860	100	56.8	LOS E	18.8	147.9	Full	500	0.0	0.0
Lane 3	113	37.1	184	0.613	100	62.3	LOS E	6.6	60.6	Short	60	0.0	NA
Approach	737	20.6		0.860		59.2	LOS E	20.7	170.0				
Intersection	2846	9.2		0.861		56.2	LOS E	26.9	195.2				

Level of Service (LOS) Method: Delay (HCM 2000).

Lane LOS values are based on average delay per lane.

Intersection and Approach LOS values are based on average delay for all lanes.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

1 Reduced capacity due to a short lane effect. Short lane queues may extend into the adjacent full-length lanes. Some upstream delays at entry to short lanes are not included.

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PHASING SUMMARY

Site: Traffic Signals - AM

New Site Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time)

Phase times determined by the program Sequence: Leading Right Turn (phase reduction applied) Movement Class: All Movement Classes Input Sequence: A, B, C, C1, D Output Sequence: A, B, C, D

Phase Timing Results

Phase	Α	В	С	D
Reference Phase	No	Yes	No	No
Phase Change Time (sec)	92	0	38	59
Green Time (sec)	22	32	15	27
Yellow Time (sec)	4	4	4	4
All-Red Time (sec)	2	2	2	2
Phase Time (sec)	28	38	21	33
Phase Split	23 %	32 %	18 %	28 %



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Site: Traffic Signals - PM

New Site

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time)

Move	mont Porf	ormance - \	/ohiclos								
Mov ID	OD Mov	Demance - N Demanc Total		Deg. Satn	Average Delay	Level of Service	95% Back Vehicles		Prop.	Effective	Average
U	IVIOV	veh/h	пv %	v/c	sec	Service	venicies veh	Distance m	Queued	Stop Rate per veh	Speed km/t
South	: Hawdon S										
1	L2	101	2.5	0.889	65.4	LOS E	25.9	184.0	1.00	1.03	29.5
2	T1	604	1.6	0.889	59.9	LOS E	25.9	184.0	1.00	1.04	30.0
3	R2	78	14.3	0.889	65.8	LOS E	25.6	185.3	1.00	1.04	29.5
Appro	bach	783	3.0	0.889	61.2	LOS E	25.9	185.3	1.00	1.04	29.9
East:	New Dookie	Road									
4	L2	127	1.4	0.753	53.0	LOS D	19.2	137.9	0.99	0.88	32.6
5	T1	560	4.0	0.753	47.4	LOS D	19.4	140.6	0.99	0.88	33.5
6	R2	347	1.2	0.905	70.6	LOS E	23.6	167.1	1.00	0.98	27.4
Appro	bach	1035	2.7	0.905	55.9	LOS E	23.6	167.1	0.99	0.92	31.
North	: Verney Roa	ad									
7	L2	201	4.1	0.875	64.1	LOS E	24.5	175.3	1.00	1.09	29.8
8	T1	466	1.3	0.875	58.8	LOS E	24.5	175.3	1.00	1.05	30.1
9	R2	101	0.0	0.875	64.7	LOS E	23.4	165.0	1.00	1.02	29.8
Appro	bach	768	1.9	0.875	61.0	LOS E	24.5	175.3	1.00	1.05	30.0
West:	Balaclava F	Road									
10	L2	114	3.1	0.914	76.4	LOS E	16.4	120.3	1.00	1.07	26.8
11	T1	365	7.4	0.914	70.8	LOS E	16.6	123.6	1.00	1.07	27.6
12	R2	145	0.0	0.722	65.1	LOS E	8.7	61.2	1.00	0.85	28.6
Appro	bach	624	4.9	0.914	70.5	LOS E	16.6	123.6	1.00	1.02	27.7
All Ve	hicles	3211	3.0	0.914	61.2	LOS E	25.9	185.3	1.00	1.00	29.8

Level of Service (LOS) Method: Delay (HCM 2000).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Mov		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P1	South Full Crossing	53	42.6	LOS E	0.2	0.2	0.84	0.84
P2	East Full Crossing	53	47.8	LOS E	0.2	0.2	0.89	0.89
P3	North Full Crossing	53	53.3	LOS E	0.2	0.2	0.94	0.94
P4	West Full Crossing	53	46.0	LOS E	0.2	0.2	0.88	0.88
All Pe	destrians	211	47.4	LOS E			0.89	0.89

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: Traffic Signals - PM

New Site

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time)

Lane Use			e						(A			-	
	Demand F Total	lows- HV	Cap.	Deg. Satn	Lane Util.	Average Delav	Level of Service	95% Back o Veh	t Queue Dist	Lane Config	Lane Length	Cap. Adj.	Prob. Block.
	veh/h	пv %	veh/h	V/C	011. %	Sec	Service	ven	m	Coning	m	Auj. %	ыоск. %
South: Haw		/0	Veniim	10	70							/0	/0
Lane 1	394	1.8	444	0.889	100	61.3	LOS E	25.9	184.0	Short	150	0.0	NA
Lane 2	389	4.1	438	0.889	100	61.2	LOS E	25.6	185.3	Full	500	0.0	0.0
Approach	783	3.0		0.889		61.2	LOS E	25.9	185.3				
East: New Dookie Road													
Lane 1	342	3.0	454	0.753	100	49.5	LOS D	19.2	137.9	Short	200	0.0	NA
Lane 2	346	4.0	459	0.753	100	47.3	LOS D	19.4	140.6	Full	500	0.0	0.0
Lane 3	347	1.2	384	0.905	100	70.6	LOS E	23.6	167.1	Short	120	0.0	NA
Approach	1035	2.7		0.905		55.9	LOS E	23.6	167.1				
North: Verne	ey Road												
Lane 1	406	2.7	464	0.875	100	61.2	LOS E	24.5	175.3	Short	200	0.0	NA
Lane 2	363	0.9	414	0.875	100	60.7	LOS E	23.4	165.0	Full	500	0.0	0.0
Approach	768	1.9		0.875		61.0	LOS E	24.5	175.3				
West: Balac	lava Road												
Lane 1	238	5.3	261	0.914	100	73.5	LOS E	16.4	120.3	Short	150	0.0	NA
Lane 2	241	7.4	264	0.914	100	70.7	LOS E	16.6	123.6	Full	500	0.0	0.0
Lane 3	145	0.0	201	0.722	100	65.1	LOS E	8.7	61.2	Short	60	0.0	NA
Approach	624	4.9		0.914		70.5	LOS E	16.6	123.6				
Intersection	3211	3.0		0.914		61.2	LOS E	25.9	185.3				

Level of Service (LOS) Method: Delay (HCM 2000).

Lane LOS values are based on average delay per lane.

Intersection and Approach LOS values are based on average delay for all lanes.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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PHASING SUMMARY

Site: Traffic Signals - PM

New Site Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time)

Phase times determined by the program Sequence: Leading Right Turn Movement Class: All Movement Classes Input Sequence: A, B, C, C1, D Output Sequence: A, B, C, C1, D

Phase Timing Results

Phase	Α	В	С	C1	D
Reference Phase	No	Yes	No	No	No
Phase Change Time (sec)	86	0	32	51	63
Green Time (sec)	28	26	13	6	17
Yellow Time (sec)	4	4	4	4	4
All-Red Time (sec)	2	2	2	2	2
Phase Time (sec)	34	32	19	12	23
Phase Split	28 %	27 %	16 %	10 %	19 %



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